

Given $f_i: \Omega \rightarrow \mathbb{R}$, $g_i: \Gamma_{g_i} \rightarrow \mathbb{R}$ and $h_i: \Gamma_{h_i} \rightarrow \mathbb{R}$, find $u_i \in S_i$ such that

$$\int_{\Omega} w_{(i,j)} T_{ij} d\Omega = \int_{\Omega} w_i f_i d\Omega + \sum_{\alpha=1}^{n_{sd}} \left(\int_{\Gamma_{h_i}} w_{\alpha} h_{\alpha} d\Gamma \right) H(w_i)_i$$

This sum is integer

with $T_{ij} = C_{ijkl} E_{kl}$, $E_{kl} = U_{(i,j)} = \frac{u_{i,j} + u_{j,i}}{2}$

$$S_i = \{u_i \mid u_i \in H^1, u_i = g_i \text{ on } \Gamma_{g_i}\} \quad i=1(\text{l}) n_{sd}$$

$$D_i = \{w_i \mid w_i \in H^1, w_i = 0 \text{ on } \Gamma_{g_i}\} \quad i=1(\text{l}) n_{sd}$$

1. Review text discussion of $(S) \rightarrow (W), (W) \rightarrow (S)$

note- $\int_{\Omega} w_{(i,j)} T_{ij} d\Omega = \int_{\Omega} w_{(i,j)} C_{ijkl} E_{kl} d\Omega$

Stated with abstract form -

Given f, g and h (defined over right stuff)

find $\tilde{u} \in \tilde{S}$ such that

$$a(\tilde{w}, \tilde{u}) = (\tilde{w}, f) + (\tilde{w}, h) \quad \forall \tilde{w} \in \tilde{V}$$

where

$$a(\tilde{w}, \tilde{u}) = \int_{\Omega} w_{(i,j)} C_{ijkl} E_{kl} d\Omega$$

$$(\tilde{w}, f) = \int_{\Omega} w_i f_i d\Omega$$

$$(\tilde{w}, h) = \sum_{\alpha=1}^{n_{sd}} \int_{\Gamma_{h_i}} w_{\alpha} h_{\alpha} d\Gamma$$

no sum.

We now work toward a "convenient" matrix description

we begin by representing the necessary terms from our second order tensors as terms listed in a vector-

(they still are second order tensors and transform coordinates like second order tensors - (not like vectors))

recalling that $\tau_{ij} = \tau_{ji}$

for $n_{sd}=2$ $\tau_{12} = \tau_{21}$, so only 3 independent terms $\left\{ \tau_{11}, \tau_{22}, \text{ and } \tau_{12} (\text{or } \tau_{21}) \right\}$

Thus we will define

$$n_{sd}=2 \quad \tilde{\tau} = \begin{Bmatrix} \tau_{11} \\ \tau_{22} \\ \tau_{12} \end{Bmatrix}$$

for $n_{sd}=3$ we have $\tau_{12} = \tau_{21}, \tau_{13} = \tau_{31}, \tau_{23} = \tau_{32}$
so $\tau_{11}, \tau_{22}, \tau_{33}, \tau_{12}, \tau_{13}, \text{ and } \tau_{23}$ can be the 6 independent terms

The text book ordering of the 6 terms

$$n_{sd}=3 \quad \tilde{\tau} = \begin{Bmatrix} \tau_{11} \\ \tau_{22} \\ \tau_{33} \\ \tau_{23} \\ \tau_{13} \\ \tau_{12} \end{Bmatrix}$$

Like wise we define a 3×1 strain vector for $n_{sd}=2$ and 6×1 strain vector for 3D
How we convert from Green's strains to "Engineering strains" (double "shear" terms).

$$n_{sd}=2 \quad \underline{\underline{\epsilon}}(u) = \{ \underline{\epsilon}_I(u) \} = \begin{Bmatrix} u_1 \\ u_{2,2} \\ u_{1,2} + u_{2,1} \end{Bmatrix}$$

$$n_{sd}=3 \quad \underline{\underline{\epsilon}}(u) = \{ \underline{\epsilon}_I(u) \} = \begin{Bmatrix} u_{1,1} \\ u_{2,2} \\ u_{3,3} \\ u_{2,3} + u_{3,2} \\ u_{1,3} + u_{3,1} \\ u_{1,2} + u_{2,1} \end{Bmatrix}$$

We need our constitutive eq. in terms of the new stress and strain vectors

$$\underline{\underline{\sigma}} = \underline{\underline{\Omega}} \underline{\underline{\epsilon}}(u) \quad , \quad \{ \underline{\sigma} \} = [D] \{ \underline{\epsilon}(u) \}$$

$$[D] = [D]^T \quad \text{symmetric} \quad e = n_{sd}(n_{sd}+1)/2 \quad , \quad e = 3 \text{ for } n_{sd}=2 \quad , \quad e = 6 \text{ for } n_{sd}=3$$

If we are given the terms C_{ijkl} using the known symmetries of C_{ijkl} and $\underline{\underline{\Omega}}$ as well as the relations from σ_{ij} to $\underline{\underline{\sigma}}$ and ϵ_{ij} to $\underline{\underline{\epsilon}}(u)$ we can construct terms in D .

Text book shows how to do this

Since no one does this - I will not cover it or ask you to do it on home work or test.

with all this done we have

$$w_{(i,j)} C_{ijkl} u_{(k,l)} = \underbrace{\varepsilon(w)^T}_{\sim} \underbrace{Q}_{\sim} \underbrace{\varepsilon(u)}_{\sim}$$

$$a(\underline{w}, \underline{u}) = \int_R \underbrace{\varepsilon(w)^T}_{\sim} \underbrace{Q}_{\sim} \underbrace{\varepsilon(u)}_{\sim} dR$$

Doing our normal decomposition of
 $\underline{u}^h = \underline{v}^h + \underline{g}^h$, $\underline{u}^h \in S^h$, $\underline{v}^h \in V^h$, $\underline{g}^h \in S^h$

where:

$$V^h = \{w_i^h | w_i^h \in V_i^h\}, V_i^h = \{w_i^h | w_i^h \in H^1, w_i^h = 0 \text{ on } \Gamma_{g_i}\}$$

$$S^h = \{u_i^h | u_i^h \in S_i^h\}, S_i^h = \{u_i^h | u_i^h \in H^1, u_i^h = g_i \text{ on } \Gamma_{g_i}\}$$

we have our Galerkin form

Given \underline{f} , \underline{g} and \underline{h} as before find $\underline{u}^h = \underline{v}^h + \underline{g}^h \in S^h$
such that

$$a(\underline{w}^h, \underline{v}^h) = (\underline{w}^h, \underline{f}) + (\underline{w}^h, \underline{h})_p - a(\underline{w}^h, \underline{g}^h)$$

To get to the matrix form we have
to deal with the fact that essential BC can
be associated with any nodes (in
general dof holders)

(ndof \geq nsel
for vector
system)

Assuming nodal dof only where each
node can have n_{dof} ,
lets use $ndof = nsd$ for the basic vector system

in this case the total possible number of dof is

$$n_{DOF} = n_{dof} n_{np} = n_{sd} n_{np} \text{ for our current case}$$

$\underbrace{\# \text{ of nodes}}$

n_{eq} = actual # dof in global system

$n_{eq} < n_{DOF} \Leftarrow$ must have sufficient essential B.C.

Consider again the set $\mathcal{N} = \{1, 2, 3, \dots, n_{np}\}$
and now define set

$$\mathcal{N}_{g_i} = \{\text{nodes with a } g_i\}$$

with this we can write

$$v_i^h = \sum_{A \in \mathcal{N} - \mathcal{N}_{g_i}} N_A d_{LA} \quad i=1(1)n_{sd}$$

$$g_i = \sum_{A \in \mathcal{N}_{g_i}} N_A g_{iA} \quad i=1(1)n_{sd}$$

Introducing yet another construct:

Euclidean basis vector e_i

$$n_{sd}=2 \quad \tilde{e}_1 = \begin{cases} 1 \\ 0 \end{cases}, \quad \tilde{e}_2 = \begin{cases} 0 \\ 1 \end{cases}$$

$$n_{sd}=3 \quad \tilde{e}_1 = \begin{cases} 1 \\ 0 \\ 0 \end{cases}, \quad \tilde{e}_2 = \begin{cases} 0 \\ 1 \\ 0 \end{cases}, \quad \tilde{e}_3 = \begin{cases} 0 \\ 0 \\ 1 \end{cases}$$

with this we can write

$$\underline{\underline{w}}^h = \underline{U}_i^h \underline{e}_i , \quad \underline{\underline{g}}^h = \underline{g}_i^h \underline{e}_i \quad (\text{removing sum})$$

$$\underline{\underline{w}}^h = \underline{W}_i^h \underline{e}_i , \quad \underline{W}_i^h = \sum_{A \in \mathcal{N} - \mathcal{N}_{g_i}} N_A C_{iA}$$

Note - notation still a bit messy but idea is to include the correct terms in each summation as well as accounting for components -

Substituting all this in and recalling the C_{iA} 's are arbitrary we have

$$\sum_{j=1}^{n_{sd}} \left(\sum_{B \in \mathcal{N} - \mathcal{N}_{g_i}} a(N_A \underline{e}_i, N_B \underline{e}_j) d_j B \right) =$$

(Using the
Dof = n_{sd}
case)

$$= \sum_{j=1}^{n_{sd}} \left(\sum_{B \in \mathcal{N} - \mathcal{N}_{g_i}} a(N_A \underline{e}_i, N_B \underline{e}_j) g_j B \right) \quad A \in \mathcal{N} - \mathcal{N}_{g_i} \quad 1 \leq i \leq n_{sd}$$

for writing the final matrix form

we can use the ID device - now as a matrix

$$ID(i, A)$$

$i = 1(1) n_{sd}$ (vector system)
 $A = 1(1) n_{np}$ (nodal dof only)

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P=0
Do A = 1 to nnp
  Do i = 1 to nsd
    If A ∈ n - ngi
      P=P+1
      ID(l, A)=P ← equation # in R
    else
      ID(l, A)=0
    end if
  end Do
end Do
nef=P ← total number of actual executions

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Using this we have

$$\{K\} \{d\} = \{F\} \quad (Kd=F)$$

$$P = ID(l, A), \quad Q = ID(j, B)$$

$$K_{PQ} = a(N_A e_i, N_B e_j)$$

$$F_P = (N_A e_i, f) + (N_A e_i h) r - \sum_{j=1}^{n_{sd}} \left(\sum_{B \in n_{g_j}} a(N_A e_i, N_B e_j) g_{jB} \right)$$

⇒ We will see exactly how terms go into place when we look at our pseudo code later

To complete the ability to use vector matrix form we want to introduce the "B" matrix again

$$\underline{\underline{\epsilon}}(N_A \underline{\underline{e}}_i) = \underline{\underline{B}}_A \underline{\underline{e}}_i \leftarrow \text{Strain-displacement operator}$$

for $n_{sd}=2$

$$\underline{\underline{B}}_A = \begin{bmatrix} N_{A,1} & 0 \\ 0 & N_{A,2} \\ N_{A,2} & N_{A,1} \end{bmatrix}$$

$\overset{3 \times 2}{\uparrow}$
#strain components

for $n_{sd}=3$

$$\begin{bmatrix} N_{A,1} & 0 & 0 \\ 0 & N_{A,2} & 0 \\ 0 & 0 & N_{A,3} \\ 0 & N_{A,3} & N_{A,2} \\ N_{A,3} & 0 & N_{A,1} \\ N_{A,2} & N_{A,1} & 0 \end{bmatrix}$$

6×3

With this we can write (not very useful!)

$$K_{PQ} = \underline{\underline{e}}_i^T \left\{ \underline{\underline{B}}_A^T D \underline{\underline{B}}_B d\Omega \right\} \underline{\underline{e}}_j$$

and noting that $\int \underline{\underline{N}}_A \underline{\underline{e}}_i f d\Omega = \int \underline{\underline{N}}_A f d\Omega$

$$\int \underline{\underline{N}}_A \underline{\underline{e}}_i h d\Omega = \int \underline{\underline{N}}_A h d\Omega$$

II-27

$$F_p = \int_n N_A f_i d\Gamma + \int_{\Gamma_{hi}} N_A h_i d\Gamma - \sum_{j=1}^{n_{sd}} \left(\sum_{B \in g_j} \alpha(N_A e_i, N_B e_j) g_j B \right)$$

still messy -

Finally - We do not want to do those crazy integrals to directly get K_{PQ} and F_P terms

We want to do elements - the whole element - at once and have the assembly operator take care of putting things in the right place the right way

$$\underline{\underline{K}} = \sum_{e=1}^{N_{\text{el}}} \underline{\underline{k}}^e, \quad \underline{\underline{F}} = \sum_{e=1}^{N_{\text{el}}} (\underline{\underline{k}}^e \underline{f}^e)$$

N_{en} = # nodes / element

N_{ed} = # dof / node = N_{sd} for simple vector systems

$N_{\text{ee}} = N_{\text{en}} N_{\text{ed}} = \# \text{ of dof of the element}$
 \rightarrow will be the size of element $\underline{\underline{k}}^e, \underline{f}^e$

$$\underline{\underline{k}}^e = [k_{pq}^e], \quad \underline{f}^e = \{f_{pq}\} \quad 1 \leq p, q \leq N_{\text{ee}}$$

$$k_{pq} = \int_{\Omega_e} B_a^T D B_b d\Omega_e \quad p = N_{\text{ed}}(a-1) + i \\ q = N_{\text{ed}}(b-1) + j$$

for $N_{\text{sd}} = 2$

$$\underline{B}_a = \begin{bmatrix} N_{a,1} & 0 \\ 0 & N_{a,2} \\ N_{a,2} & N_{a,1} \end{bmatrix}$$

for

$$N_{\text{sd}} = 3 \quad \underline{B}_a =$$

$$\begin{bmatrix} N_{a,1} & 0 & 0 \\ 0 & N_{a,2} & 0 \\ 0 & 0 & N_{a,3} \\ 0 & N_{a,3} & N_{a,2} \\ N_{a,3} & 0 & N_{a,1} \\ N_{a,2} & N_{a,1} & 0 \end{bmatrix}$$

what I called \tilde{f}_p^e

$$\tilde{f}_p^e = \int_{\Gamma_e} N_a f_i dr + \int_{\Gamma_e} N_a h_i d\Gamma - \sum_{g=1}^{n_{ee}} k_{pg} g_g^e$$

$$\Gamma_{hi}^e = \Gamma_{hi} \cap \Gamma^e$$

portion of elements

$g_g = g_j(\chi_b^e)$ if node is on Γ_{gj}
is zero otherwise

In actual programs we avoid the \tilde{e}_i 's
and just do the whole thing at once
for example -

Define $B = [B_1, B_2, \dots, B_{n_{en}}]$

then

$$\tilde{K}^e = \int_{\Gamma_e} B^T D B dr$$

Stress evaluation $\tilde{\epsilon}(u)$ = $\sum_{a=1}^{n_{en}} B_a \tilde{d}_a$ dof at a node

$$\tilde{\sigma}(x) = D(x) \tilde{\epsilon}(u(x)) = D(x) \sum_{a=1}^{n_{en}} B_a \tilde{d}_a$$

$$= D(x) B \tilde{d}^e$$

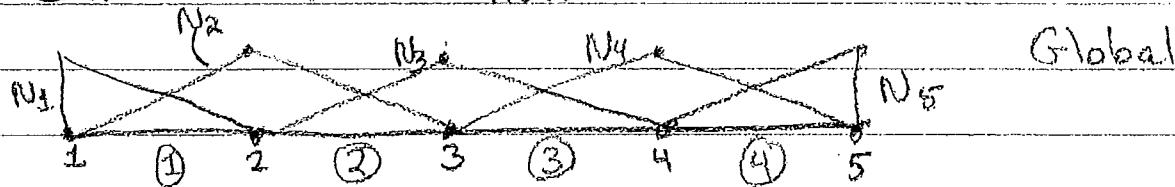
where

$$B = [B_1, B_2, B_3, \dots, B_{n_{en}}], \quad \tilde{d} = \begin{Bmatrix} \tilde{d}_1^e \\ \tilde{d}_2^e \\ \vdots \\ \tilde{d}_{n_{en}}^e \end{Bmatrix}$$

(C)

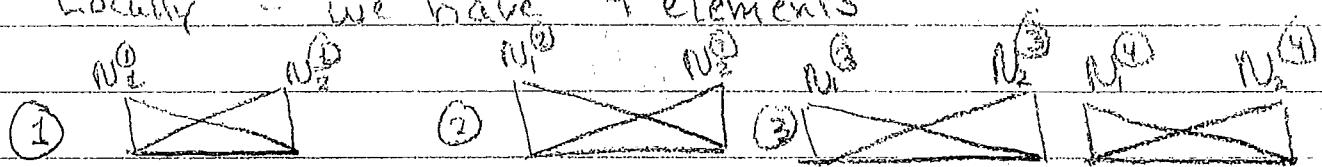
Global dof - sum element contributions

Let's look at this for our specific case with 5 nodes



$n=4$, with essential BC on right need
One basis function for that $\rightarrow N_5 \leftarrow N_{n+1}$

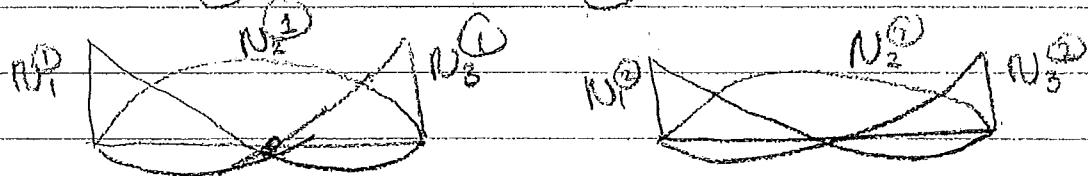
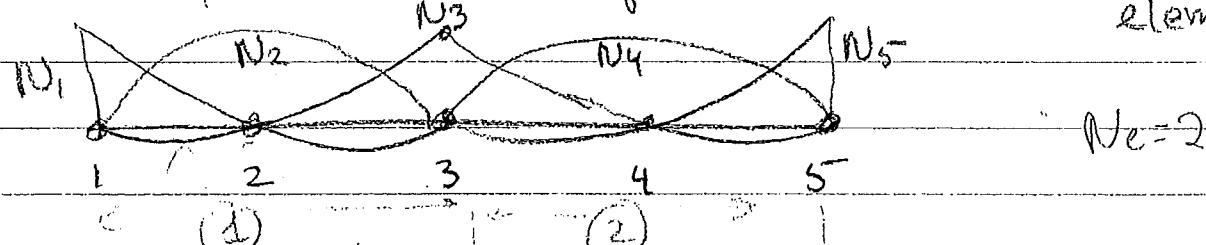
Locally - we have 4 elements



$$N_1 = N_1^{(1)}, N_2 = N_2^{(1)} + N_2^{(2)}, N_3 = N_2^{(2)} + N_3^{(1)}$$

$$N_4 = N_3^{(2)} + N_4^{(1)}, \quad N_5 = N_4^{(1)}$$

Can also do the quadratic \leftarrow Now 2-3 nodal elements

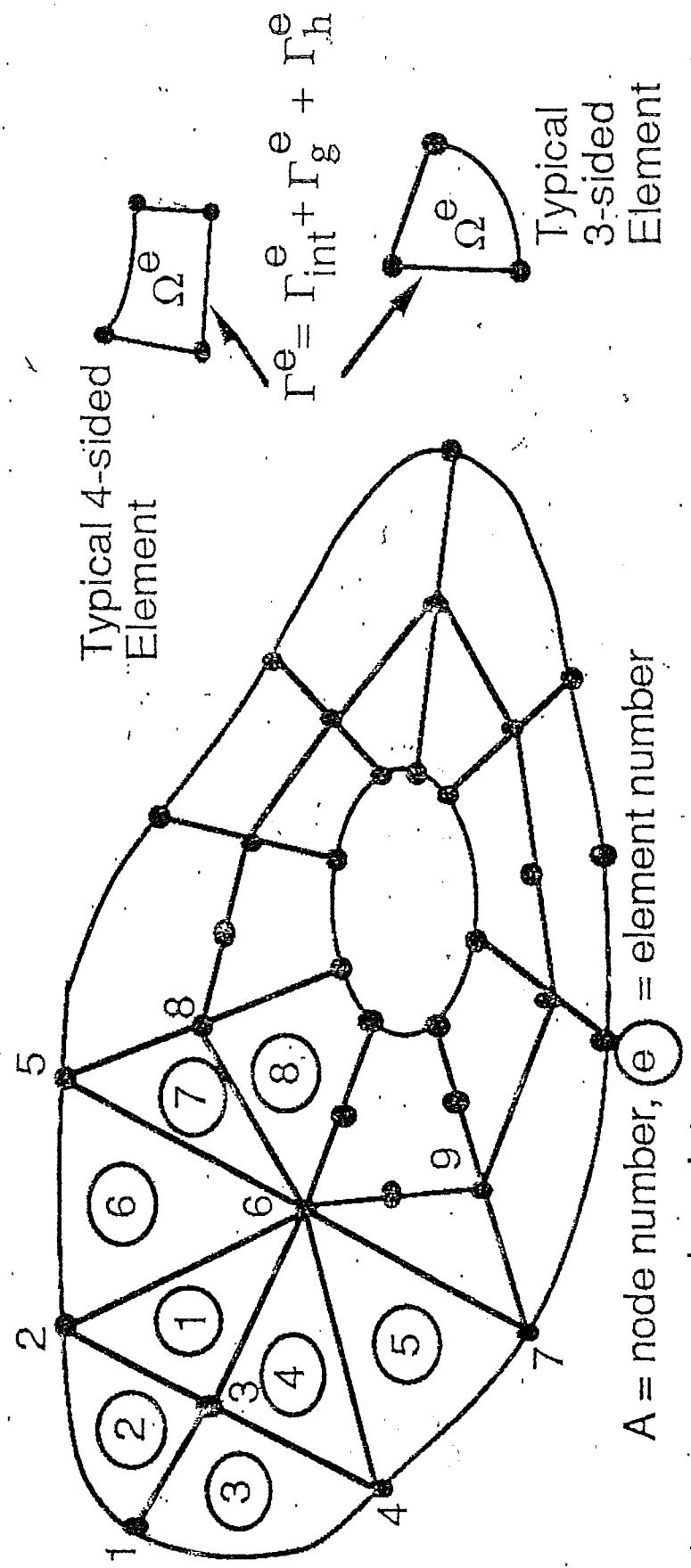


$$N_1 = N_1^{(1)}, N_2 = N_2^{(1)}, N_3 = N_3^{(1)} + N_3^{(2)}$$

$$N_4 = N_2^{(2)}, \quad N_5 = N_3^{(2)}$$

Note - each shape function meets $(N_A(x_B)) = \delta_{AB}$

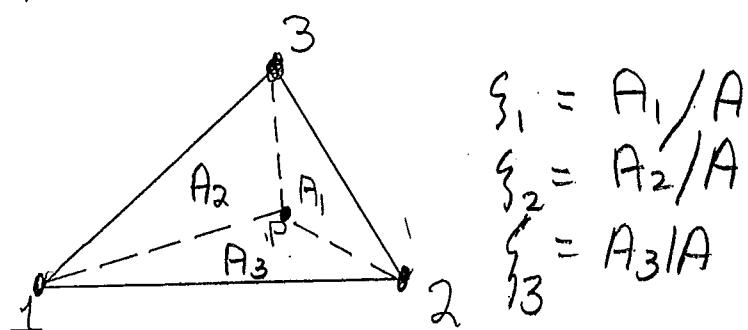
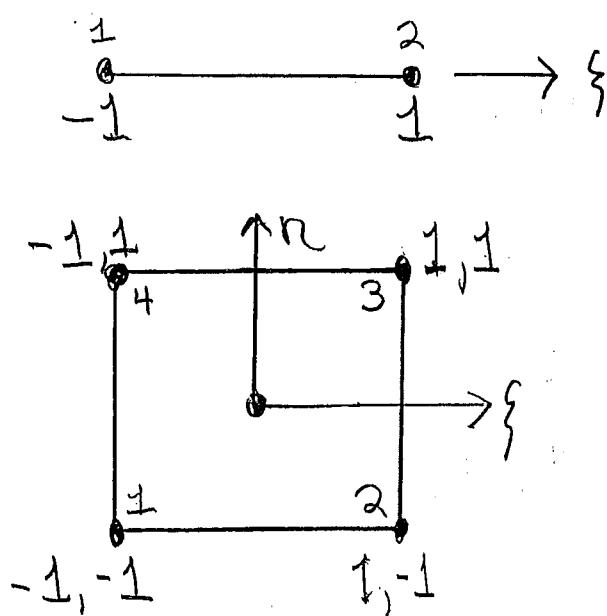
shape functions are continuous in value, but not slope
between elements



Local Coordinates -

It is not convenient to write our shape functions directly in the global coordinate system.

We prefer to write them in a local coordinate system and then deal with the mapping required

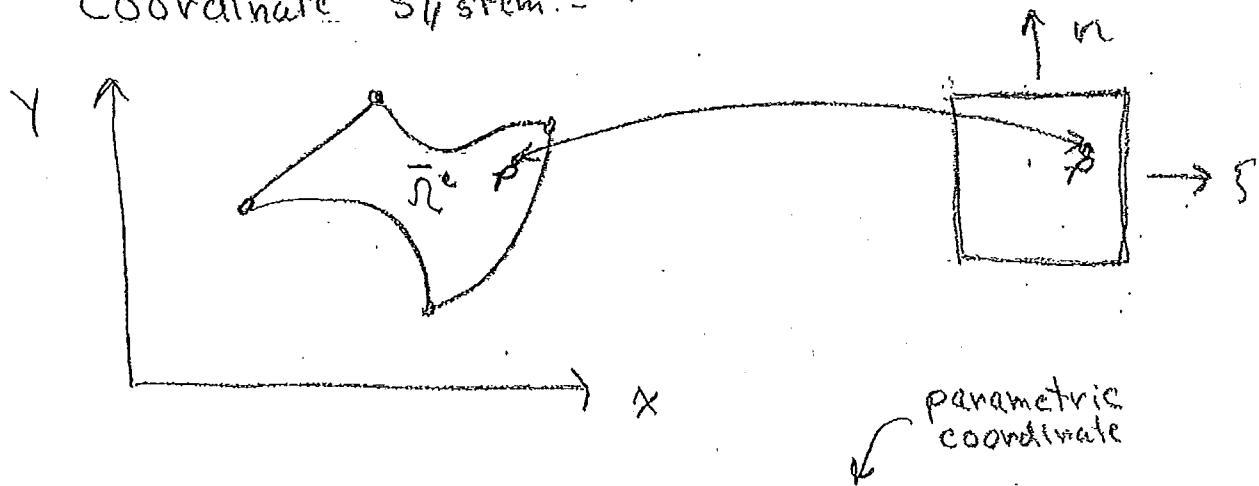


Subject to: $\xi_1 + \xi_2 + \xi_3 = \frac{A_1 + A_2 + A_3}{A} = \frac{A}{A} = 1$

Use - $\xi_3 = 1 - \xi_1 - \xi_2$, $\xi_2 = 1 - \xi_1 - \xi_3$, or $\xi_1 = 1 - \xi_2 - \xi_3$

Lets revisit Mappings - a bit more general view

Relating our elements with shape functions defined in a parametric coordinate system with the element in the real (global) coordinate system.



need mapping acting on \square to relate it to global coordinates -

$$\tilde{x}: \square \rightarrow \bar{x}^e \quad \begin{matrix} \text{going from parametric} \\ \text{to real} \end{matrix}$$

We will assume elements are 1-to-1 \Leftrightarrow To be discussed later

A useful mapping shown will be -

$$\tilde{x} = Q^e(s, r)$$

- Common to define on a component by component basis -

$$x = Q_x^e(s, r) \quad ; \quad y = Q_y^e(s, r)$$

Now during the process of defining the stiffness matrix we need - $d\Omega$

$$\frac{\partial}{\partial s}, \frac{\partial}{\partial r} \quad \text{and} \quad \int f(s, r) d\Omega$$

These are some basics - still need to have something to proceed -

Consider for now elements where we use nodal values of the components of \mathbf{u} as unknowns

$$\text{Non} \quad \mathbf{u} = \sum_{a=1}^{N_a} N_a \mathbf{d}_a$$

$$u_i = \sum_{a=1}^{N_a} N_a d_{ia} \quad \text{where } \text{div}_i = u_i$$

$$\mathbf{x} = \mathbf{x}_a$$

Could take the same type of approach - That is assume we define the set of geometry nodes for which we know the coordinates of the nodes in both \mathbf{x} and \mathbf{f} \leftarrow draw some pictures \leftarrow heart \uparrow page

define N_{gen} as number of geometric nodes. Then N_{gen}

$$\mathbf{x} = \sum_{a=1}^{N_{\text{gen}}} N_a \mathbf{x}_a \quad \text{where } \mathbf{x}_a^e = \mathbf{x}_{\text{node}}^e$$

$$x_i = \sum_{a=1}^{N_{\text{gen}}} N_a x_{ia} \quad \forall i \in \mathbf{x}$$

The most common choice of N_a is $N_a = N_a$ isoparametric

Define p^g as polynomial order of N_a^g

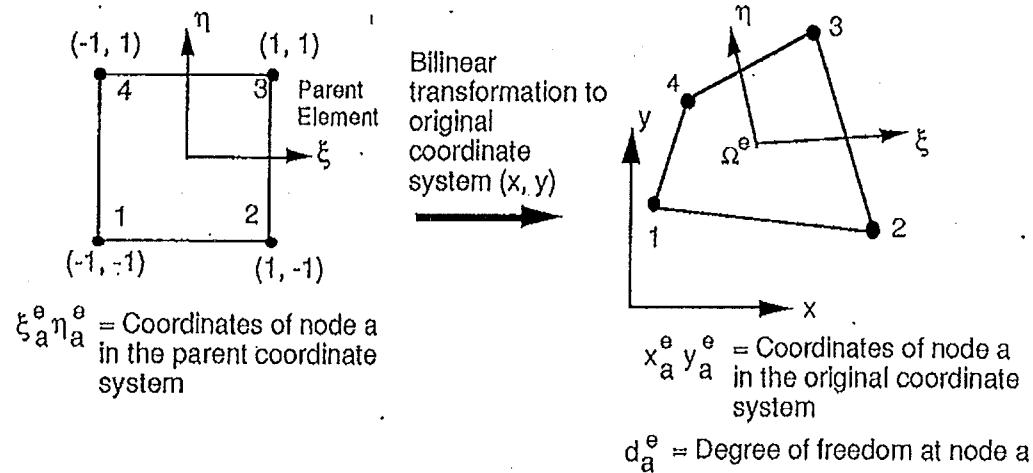
" p as polynomial order of N_a

If $p^g > p$ superparametric element - does not exactly meet constant strain state part of

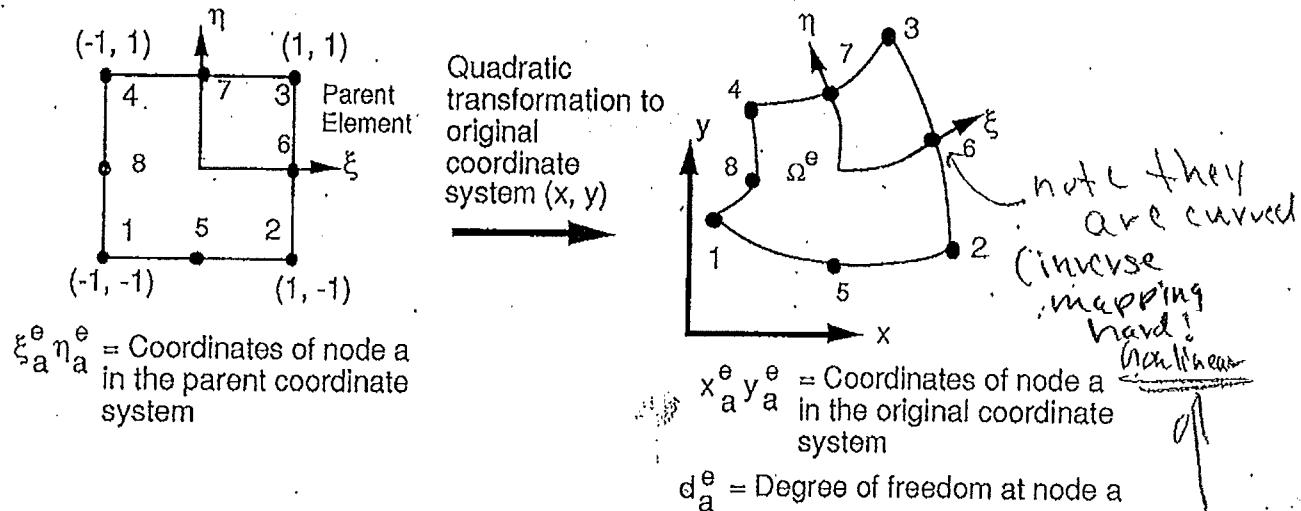
Isoparametric

- These interpolations map the square element in the parent ξ - η system into a more generally shaped element in x - y space whose shape depends on the interpolations, N_a , used.

For example, the linear (bilinear) interpolation maps the square into a quadrilateral with straight sides:



The quadratic (or biquadratic) interpolation maps the square into a quadrilateral with curved (quadratic) sides:



3-D elements add a third coordinate

so is
the
bilinear
with one
more dim

Note from before for iso-parametric

$$\tilde{X} = \sum_{a=1}^{n_m} N_a \tilde{X}_a \quad \text{and by component}$$

$$X_i = \sum_{a=1}^{n_m} N_a X_{ia}^e \quad \text{or} \quad \begin{aligned} X &= \sum_{a=1}^{n_m} N_a X_a^e \\ Y &= \sum_{a=1}^{n_m} N_a Y_a^e \end{aligned}$$

Using these last two we can rewrite

$$\frac{\partial}{\partial x} = \frac{1}{|J|} \left(\frac{\partial Y}{\partial n} \frac{\partial}{\partial s} - \frac{\partial Y}{\partial s} \frac{\partial}{\partial n} \right) =$$

$$\frac{\partial}{\partial x} = \frac{1}{|J|} \left(\frac{\partial X}{\partial n} \frac{\partial}{\partial s} + \frac{\partial X}{\partial s} \frac{\partial}{\partial n} \right)$$

$$|J| = \frac{\partial X}{\partial s} \frac{\partial Y}{\partial n} - \frac{\partial X}{\partial n} \frac{\partial Y}{\partial s}$$

Let's look at integration a bit closer
consider 2D

$$\int_{\Omega} f(x, y) d\Omega = \int_{-1}^1 \int_{-1}^1 f(\xi, \eta) J(\xi, \eta) / |\mathcal{J}| d\xi d\eta$$

More specifically consider the element stiffness matrix

$$k_e = \int_{\Omega} B^T D B d\Omega = \int_{\Omega} N_{a,x} D N_{a,x} dx$$

$B = [B_1, B_2 \dots B_n]^\top$

$N_a(\xi)$

for heat conduction

$$B_a = \begin{bmatrix} N_{a,x} \\ N_{a,y} \end{bmatrix}$$

$D = \{d\}_{2 \times 2}$
(assume it's constant)

for elasticity

$$B_a = \begin{bmatrix} N_{a,x} & 0 \\ 0 & N_{a,y} \\ N_{a,y} & N_{a,x} \end{bmatrix}$$

$D = \{D\}_{3 \times 3}$
(assume constant)

so a typical term in the matrix is

$$\frac{\partial N_{a,x}}{\partial x} = \frac{1}{|\mathcal{J}|} \left(\frac{\partial \xi}{\partial x} \frac{\partial N_a}{\partial \xi} - \frac{\partial \eta}{\partial x} \frac{\partial N_a}{\partial \eta} \right) = \frac{1}{|\mathcal{J}|} (y_{1,n} N_{a,5} - y_{1,5} N_{a,n})$$

$$\frac{\partial N_a}{\partial \eta} = \frac{1}{|\mathcal{J}|} \left(\frac{\partial \xi}{\partial \eta} \frac{\partial N_a}{\partial \xi} + \frac{\partial \eta}{\partial \eta} \frac{\partial N_a}{\partial \eta} \right) = \frac{1}{|\mathcal{J}|} (x_{1,n} N_{a,5} + x_{1,5} N_{a,n})$$

∴ substituting this in - gives

for heat conduction

$$\{\hat{B}_a\} = \frac{1}{|J|} \begin{Bmatrix} Y_{1n} N_{a,1} - Y_{1g} N_{a,g} \\ -X_{1n} N_{a,1} + X_{1g} N_{a,g} \end{Bmatrix} = \frac{1}{|J|} \hat{B}_a$$

for elasticity

$$\hat{B}_a = \frac{1}{|J|} \begin{Bmatrix} Y_{1n} N_{a,1} - Y_{1g} N_{a,g} & 0 \\ 0 & -X_{1n} N_{a,1} + X_{1g} N_{a,g} \\ -X_{1n} N_{a,1} + X_{1g} N_{a,g} & Y_{1n} N_{a,1} - Y_{1g} N_{a,g} \end{Bmatrix} = \frac{1}{|J|} \hat{B}_a$$

$$\hat{B} = [\hat{B}_1, \hat{B}_2, \dots, \hat{B}_{n_{el}}] = \frac{1}{|J|} [\hat{B}_1, \hat{B}_2, \dots, \hat{B}_{n_{el}}] = \frac{1}{|J|} \hat{B}$$

Substituting this in. yields the stiffness integral yields

$$k^e = \int \int \frac{1}{|J|} \hat{B}^T \Omega \frac{1}{|J|} \hat{B} |J| dS dn$$

see baby $|J| \neq 0$

$$k^e = \int \int \hat{B}^T \Omega \hat{B} \frac{1}{|J|} dS dn \quad \leftarrow \text{in general this is a variational function}$$

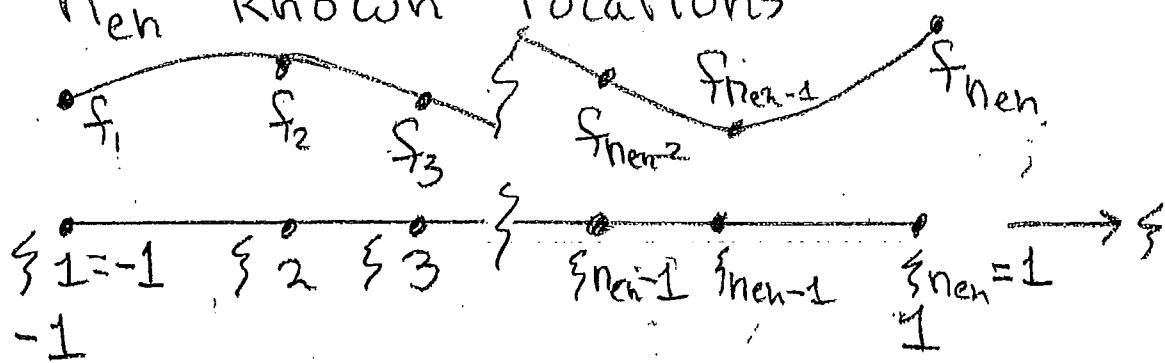
→ when the element shapes are general since $|J|$ is in that case $S(\xi, n)$ when N_a has terms greater than linear

- When it is a rational function trilize integrat~~integrate~~
is no longer given by simple formula for polynomial
interpolate appproximately

Lagrange Interpolation

A procedure to construct an order $n_{en}-1$ polynomial that interpolates a 1D function at n_{en} points

Starting point: Knowledge of the value of the function at n_{en} known locations



Lagrange Polynomials: An order $n_{en}-1$ order polynomial that interpolates at the n_{en} given locations.

$$f_e(\xi) = \sum_{a=1}^{n_{en}} l_a(\xi) f(\xi_a), \text{ note } l_a(\xi_b) = \delta_{ab}$$

$$l_a(\xi) = \frac{\prod_{\substack{b=1 \\ b \neq a}}^{n_{en}} (\xi - \xi_b)}{\prod_{\substack{b=1 \\ b \neq a}}^{n_{en}} (\xi_a - \xi_b)}$$

Two node linear



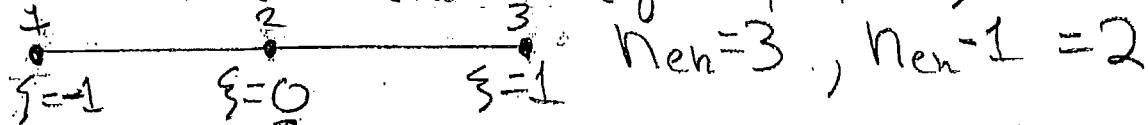
$$n_{en} = 2$$

$n_{en}-1=1 \Leftarrow$ linear functions

$$l_1'(\xi) = \frac{\prod_{\substack{b=1 \\ b \neq 1}}^2 (\xi - \xi_b)}{\prod_{\substack{b=1 \\ b \neq 1}}^2 (\xi_1 - \xi_b)} = \frac{\xi - \xi_2}{\xi_1 - \xi_2} = \frac{\xi - 1}{-1 - 1} = \frac{1}{2}(\xi - 1)$$

$$l_2'(\xi) = \frac{\prod_{\substack{b=1 \\ b \neq 2}}^2 (\xi - \xi_b)}{\prod_{\substack{b=1 \\ b \neq 2}}^2 (\xi_2 - \xi_b)} = \frac{\xi - \xi_1}{\xi_2 - \xi_1} = \frac{\xi - (-1)}{1 - (-1)} = \frac{1}{2}(\xi + 1)$$

Three Node Quadratic (equally spaced)



$$n_{en}=3, n_{en}-1=2$$

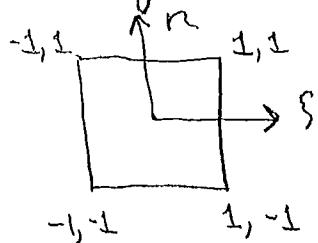
$$l_1^2(\xi) = \frac{\prod_{\substack{b=1 \\ b \neq 1}}^3 (\xi - \xi_b)}{\prod_{\substack{b=1 \\ b \neq 1}}^3 (\xi_1 - \xi_b)} = \frac{(\xi - \xi_2)(\xi - \xi_3)}{(\xi_1 - \xi_2)(\xi_1 - \xi_3)} = \frac{\xi(\xi - 1)}{(-1)(-2)} = \frac{1}{2}\xi(\xi - 1)$$

$$l_2^2(\xi) = \frac{\prod_{\substack{b=1 \\ b \neq 2}}^3 (\xi - \xi_b)}{\prod_{\substack{b=1 \\ b \neq 2}}^3 (\xi_2 - \xi_b)} = \frac{(\xi - \xi_1)(\xi - \xi_3)}{(\xi_2 - \xi_1)(\xi_2 - \xi_3)} = \frac{(\xi - (-1))(\xi - 1)}{(1)(-1)} = 1 - \xi^2$$

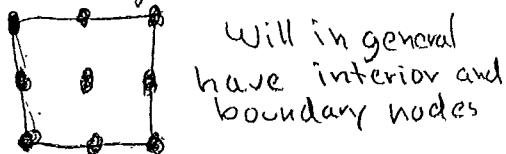
$$l_3^2(\xi) = \frac{\prod_{\substack{b=1 \\ b \neq 3}}^3 (\xi - \xi_b)}{\prod_{\substack{b=1 \\ b \neq 3}}^3 (\xi_3 - \xi_b)} = \frac{(\xi - \xi_1)(\xi - \xi_2)}{(\xi_3 - \xi_1)(\xi_3 - \xi_2)} = \frac{(\xi - (-1))(\xi - 0)}{(1 - (-1))(1 - 0)} = \frac{1}{2}\xi(\xi + 1)$$

Constructions of 2-D - 4 sided elements Using Lagrange Shape functions.

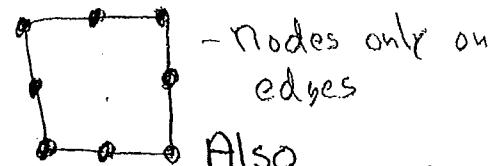
As indicated previously, the element's shape functions will be written in the parametric coordinate system - Therefore, all elements are bi-unit squares



We will consider two families of such elements
(Full) Lagrangian



Serendipity

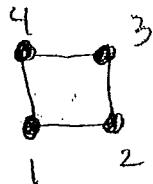


Also
Variable nodes

Start with Lagrangian -

Elements defined by multiplying 1-D Lagrange polynomials in one direction times those in the other direction(s)

Start with simple 4-noded element



With 2 nodes per side we want to use linear

Consider the 1-D Lagrange Polynomials
... and - Note - 2 nodes per edge - linear

(c) in ξ direction - $l_1^1 = \frac{1}{2}(1-\xi)$

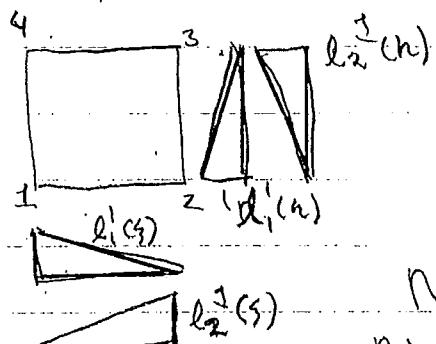
$$l_1^1 = \frac{1}{2}(1-\xi) = l_1^1(\xi)$$

$$l_2^1 = \frac{1}{2}(1+\xi) = l_2^1(\xi)$$

in n direction - $l_1^1 = \frac{1}{2}(1-n) = l_1^1(n)$

$$l_2^1 = \frac{1}{2}(1+n) = l_2^1(n)$$

the shape functions at the nodes
are the "correct" multiplication of these



$$u^h = \sum_{a=1}^4 N_a d_a$$

$$N_1 = l_1^1(s) l_1^1(n) = \frac{1}{4}(1-s)(1-n)$$

$$N_2 = l_1^1(s) l_2^1(n) = \frac{1}{4}(1-s)(1+n)$$

$$N_3 = l_2^1(s) l_1^1(n) = \frac{1}{4}(1+s)(1-n)$$

$$N_4 = l_2^1(s) l_2^1(n) = \frac{1}{4}(1+s)(1+n)$$

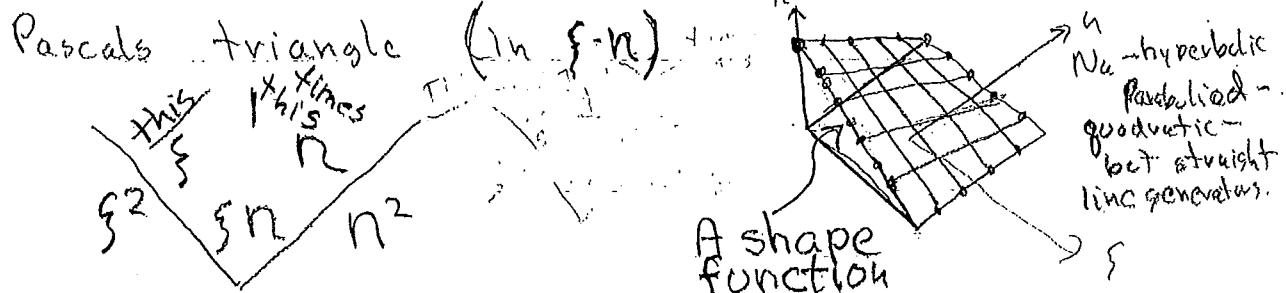
Note - $N_a = \frac{1}{4}(1+\xi_a s)(1+n_a n) = \frac{1}{4}(1+\xi_a s + n_a n + \xi_a n s)$
in other words each $a_{11} = -1$

$$N_a = a_{0a} + a_{1a}s + a_{2a}n + a_{3a}sn$$

Since each shape function has same form:

$$u^h = a_0 + a_1 s + a_2 n + a_3 sn$$

Pascals triangle



Higher Order Lagrange elements

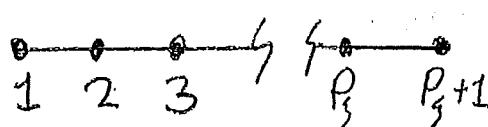
Products of 1-D shape functions

$$2D \quad N_a(\xi, n) = l_{a\xi}^{P_\xi}(\xi) l_{an}^{P_n}(n)$$

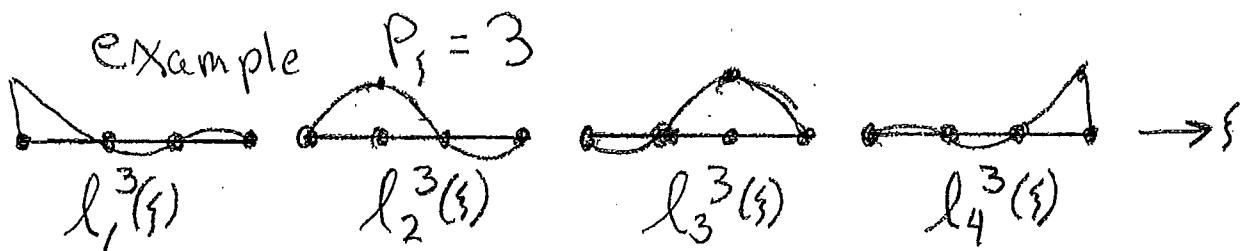
$$3D \quad N_a(\xi, n, \varsigma) = l_{a\xi}^{P_\xi}(\xi) l_{an}^{P_n}(n) l_{a\varsigma}^{P_\varsigma}(\varsigma)$$

P_ξ = polynomial order in ξ direction

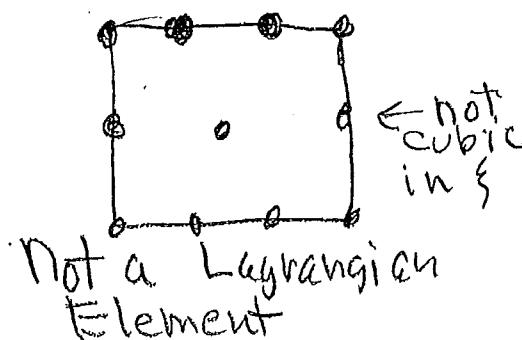
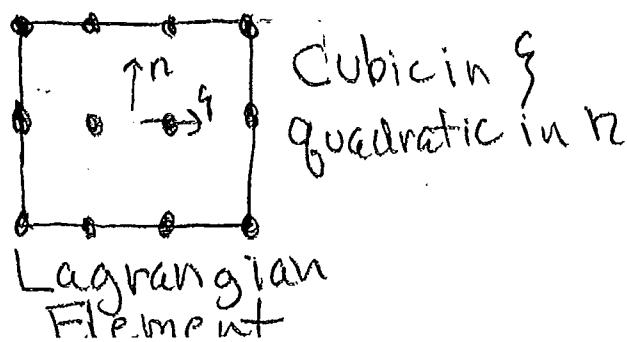
a_ξ - ξ direction "station" for node a



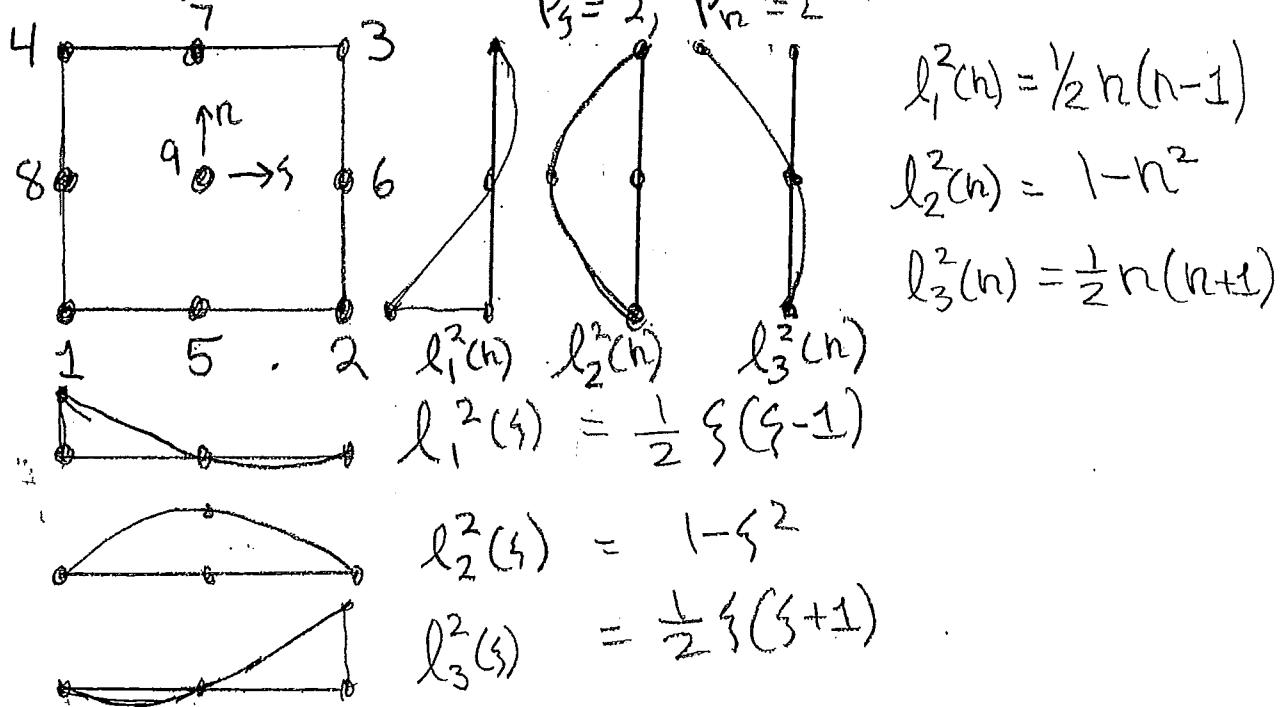
(Same for other coordinates)



Use of this formula requires the same order polynomial in a specific direction at each station in the other directions



Example - 9 noded Lagrangian Quadratic



$$\begin{array}{lll} N_1 = l_1^2(s) l_2^2(n) & N_4 = l_1^2(s) l_3^2(n) & N_7 = l_2^2(s) l_3^2(n) \\ N_2 = l_3^2(s) l_1^2(n) & N_5 = l_2^2(s) l_1^2(n) & N_8 = l_1^2(s) l_2^2(n) \\ N_3 = l_2^2(s) l_3^2(n) & N_6 = l_3^2(s) l_2^2(n) & N_9 = l_2^2(s) l_1^2(n) \end{array}$$

Conditions C1-C3 ($m=1$ case)

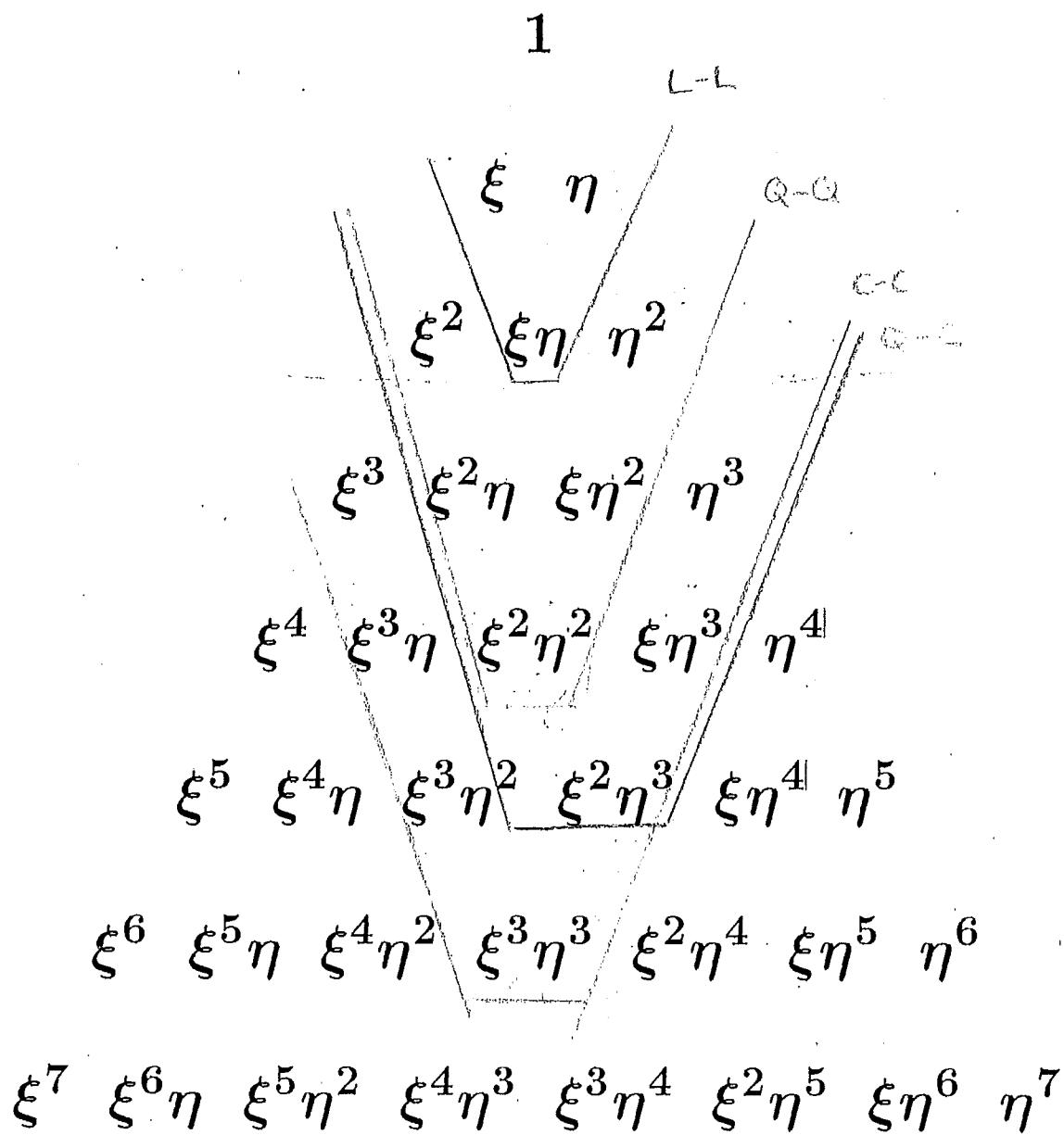
C1 - Intralelement continuity need C^1

In $s-h$, its fine / In $x-y$ also need positive J in mapping - more later

C2 - Inter element continuity = need C^0

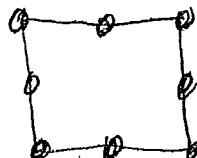
Need to show u^h is continuous between elements - Will again look at u on boundary between elements - If uniquely defined in terms of nodal values on the boundary - u^h is continuous since the nodal values are shared.

Lagrangian Elements

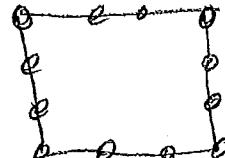


Serendipity element, variable # of nodes per edge, and other options

Serendipity - typically nodes on edges only

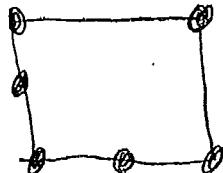


8 noded most common



12 noded

Variable # of nodes -



Useful for transition of element orders

Can not just apply products of Lagrange polynomials since those will have "zeros" that will not be accounted for - not enough conditions.

Ad-hoc construction methods required

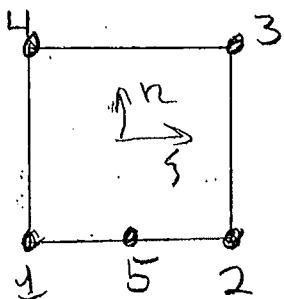
Typical procedure

1. Start with "minimal" Lagrangian element - typically will be bi-linear
2. Add proper shape functions for added nodes
3. Correct any previous shape functions to satisfy the $N_a(\xi_b) = \delta_{ab}$ condition.

easiest to simply work through examples.

\hat{N}_a - starting "minimal shape functions"
 N_a - the shape function when done

Start with easiest example - an extra node



\hat{N}_a , $a=1-4$ The bi-linear
Step 1 $\hat{N}_a = \frac{1}{4}(1+\xi_a n)(1+\xi_a s)$ $a=1(1)4$

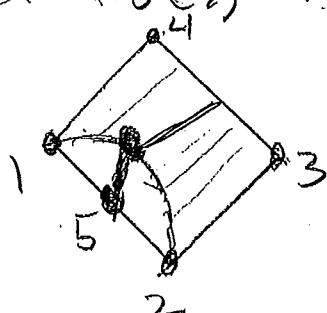
Step 2 - Define a shape function for node 5 that satisfies $N_5(\xi_{5,n}, \xi_{5,s}) = \delta_{ab}$
need function that is:

- 1 at node 5, 0 at nodes 1-4
- to be 1 at node 5 and 0 at nodes 1 and 2
need a quadratic in ξ . Note that
- $N_5|_0 = 0$ is required so in the n direction

3-4

or something

Need $N_5(\xi, -1) = 1$, $N_5(\xi, 1) = 0$



$$N_5 = \frac{1}{2}(1-\xi^2)(1-n)$$

 $l'_r(n)$

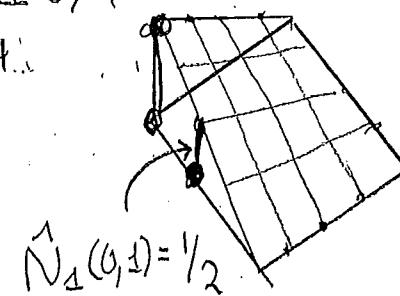
Step 3 correct any of the \hat{N}_a shape functions that are not zero at the nodes of shape functions defined in step 2

Look at $\hat{N}_{a(5,5)} \quad a=1(1)4$

$\hat{N}_{a(0,-1)} \quad a=1(1)4$

$$\hat{N}_1(0,1) = \frac{1}{2}$$

$$\hat{N}_a(0,1) = 0, \quad a=2(1)4$$



Easy to correct

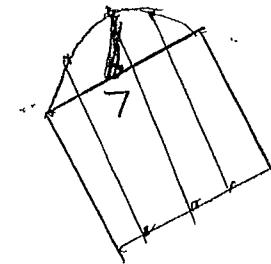
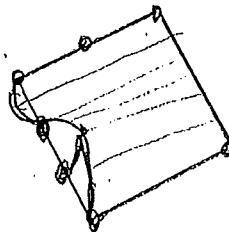
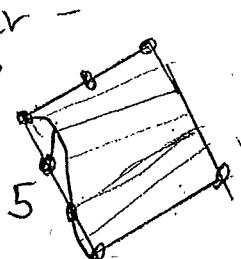
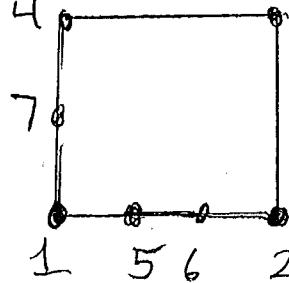
$$N_1 = \hat{N}_1 - \hat{N}_1(0,-1) N_5 = \hat{N}_1 - \frac{1}{2} N_5$$

because $N_5 = 0$ at others - causes no problem

we end up with

$$N_1 = \hat{N}_1 - \frac{1}{2} N_5, \quad N_2 = \hat{N}_2 + \frac{1}{2} N_5, \quad N_3 = \hat{N}_3, \quad N_4 = \hat{N}_4$$

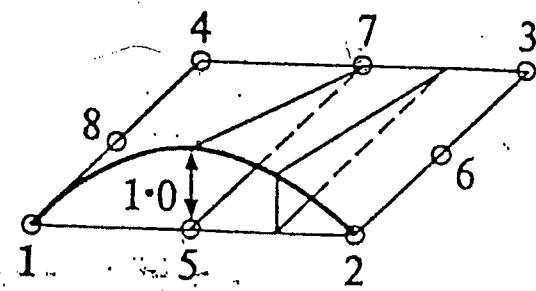
Consider -



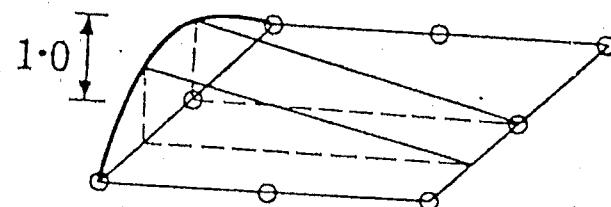
$$N_5 = l_2^3(s) l_1^1(n) \quad N_6 = l_3^3(s) l_1^1(n) \quad N_7 = l_1^1(s) l_2^2(n)$$

$$N_1 = \hat{N}_1 - \frac{2}{3} N_5 - \frac{1}{3} N_6 - \frac{1}{2} N_7, \quad N_2 = \hat{N}_2 - \frac{1}{3} N_5 - \frac{2}{3} N_6$$

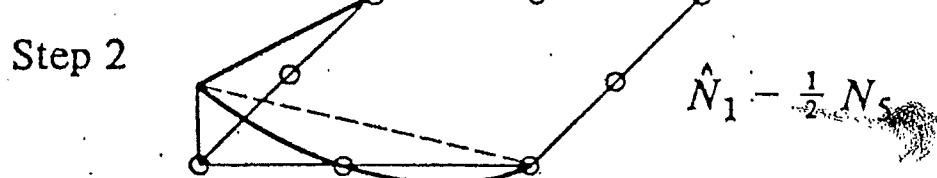
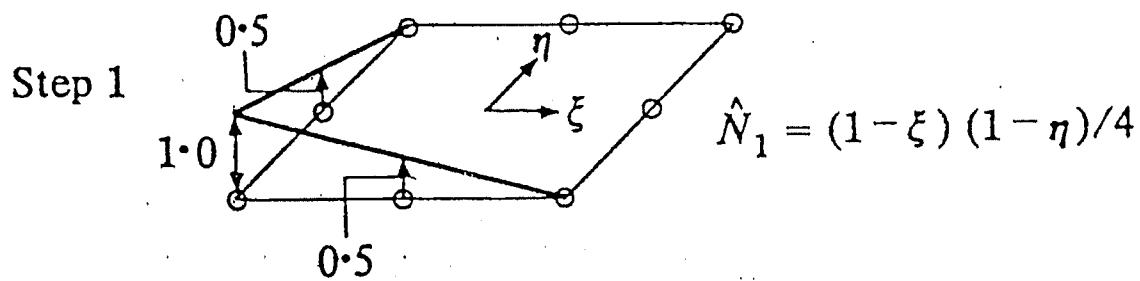
$$N_3 = \hat{N}_3, \quad N_4 = \hat{N}_4 - \frac{1}{2} N_7$$



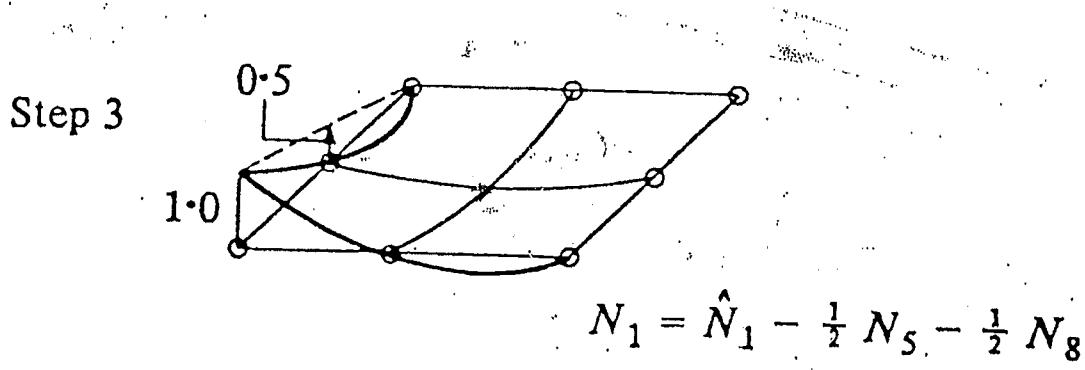
$$(a) N_5 = \frac{1}{2} (1 - \xi^2) (1 - \eta) \\ l_2^2(3) l_1^1(n)$$

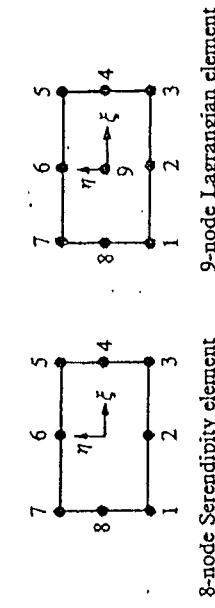


$$(b) N_8 = \frac{1}{2} (1 - \xi) (1 - \eta^2) \\ l_1^1(5) l_2^2(n)$$



(c)

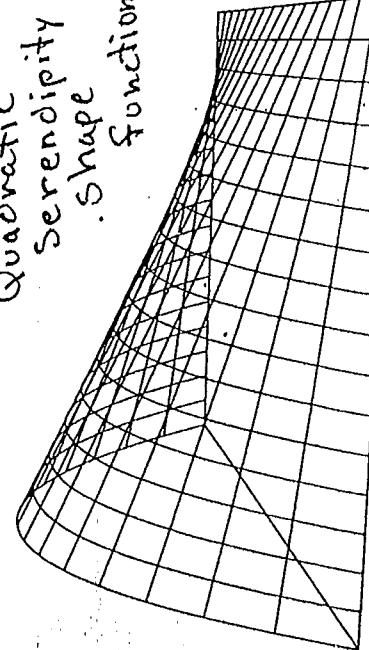




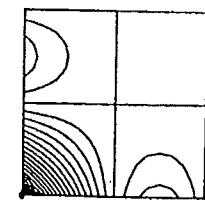
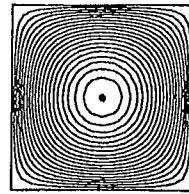
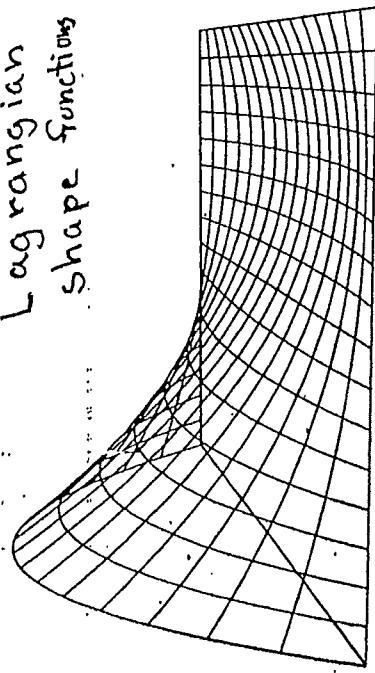
Local node number	ξ_1	η_1
1	-1	-1
2	0	-1
3	1	-1
4	1	0
5	1	1
6	0	1
7	-1	1
8	-1	0
9	0	0

Fig. 8.7 Quadratic 8 and 9-node rectangular elements.

Quadratic
Serendipity
Shape
Functions

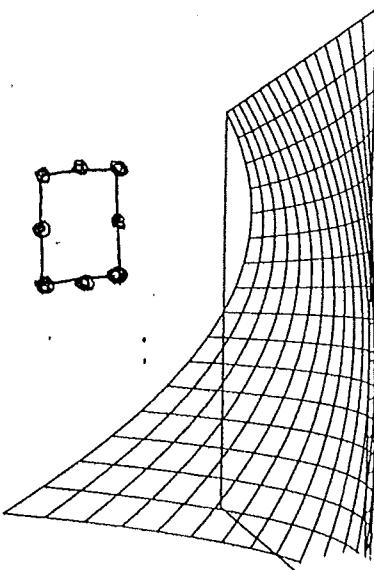
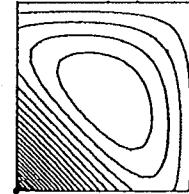
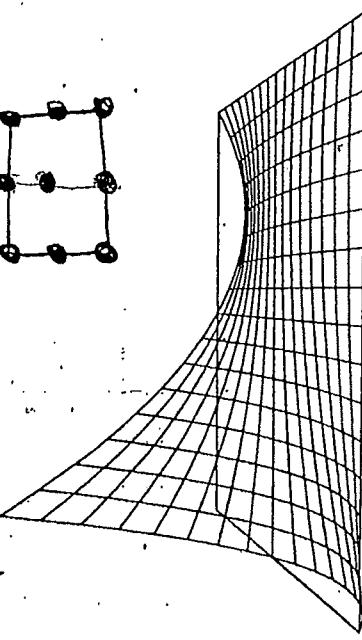


Quadratic
Lagrangean
Shape Functions



III-23

Fig. 8.10 Typical shape functions for the 9-node rectangular element.



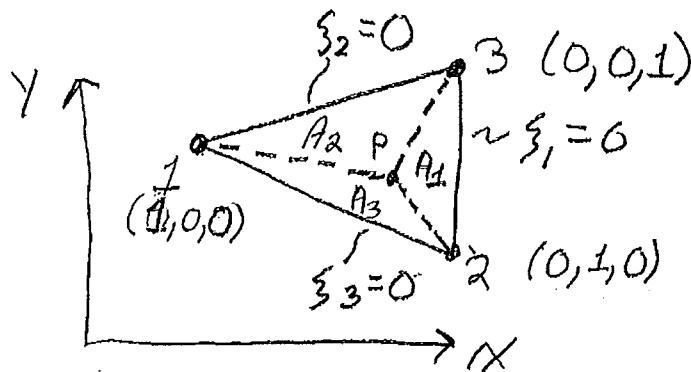
Serendipity Elements

1

$\xi \quad \eta$

ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7
ξ^7	$\xi^6\eta$	$\xi^5\eta^2$	$\xi^4\eta^3$	$\xi^3\eta^4$	$\xi^2\eta^5$	$\xi\eta^6$	η^7

Area Coordinates and Triangular elements



$$\xi_i = \frac{A_i}{A}$$

A - Area of triangle

A_i - Area of subtriangle i

$$\xi_1 + \xi_2 + \xi_3 = \frac{A_1 + A_2 + A_3}{A} = \frac{A}{A} = 1$$

Only two independent coordinates -

At some point one has to eliminate one (ex. $\xi_3 = 1 - \xi_1 - \xi_2$)

Note - The area coordinates on a straight sided element define a convenient linear interpolant

$$u = \sum_{a=1}^3 \xi_a u_a \quad da = u(X_a)$$

For an isoparametric mapping

$$x = \sum_{a=1}^3 \xi_a x_a, \quad y = \sum_{a=1}^3 \xi_a y_a$$

In the case of straight sided triangles the mapping between x and ξ is linear and easily invertable.

This will not be the case for curved elements

In the general curved case we again note that we will use a map of \tilde{x} in terms of ξ

recalling the only two of the area coordinates are independent the following is written assuming you selected ξ_1 and ξ_2 as the independent coordinates and every where there is a ξ_3 you replace it with

$$\xi_3 = 1 - \xi_1 - \xi_2$$

With this we apply chain rule knowing we can take $\frac{\partial}{\partial \xi_1}$ and $\frac{\partial}{\partial \xi_2}$ of things and invert what we have to produce

$$\frac{\partial}{\partial x} = \frac{1}{|J|} \left(\frac{\partial y}{\partial \xi_2} \frac{\partial}{\partial \xi_1} + \frac{\partial y}{\partial \xi_1} \frac{\partial}{\partial \xi_2} \right)$$

$$\frac{\partial}{\partial y} = \frac{1}{|J|} \left(- \frac{\partial x}{\partial \xi_2} \frac{\partial}{\partial \xi_1} + \frac{\partial x}{\partial \xi_1} \frac{\partial}{\partial \xi_2} \right)$$

$$|J| = \frac{\partial x}{\partial \xi_1} \frac{\partial y}{\partial \xi_2} - \frac{\partial y}{\partial \xi_1} \frac{\partial x}{\partial \xi_2}$$

We also have integration of $\int f(\xi_1, \xi_2, \xi_3) d\xi$ again using $\xi_3 = 1 - \xi_1 - \xi_2$ we have \int_{Δ}^e

$$\int_{\Delta}^e f(\xi_1, \xi_2) d\xi = \int_{\Delta}^e f(\xi_1, \xi_2) |J| d\xi_1 d\xi_2$$

parametric triangle

In the case of straight sided elements
the expressions are easier

$$|IJ| = 2A$$

and integrals of $\xi_1^\alpha \xi_2^\beta$ have closed
form expressions

$$\int_{\Delta} \xi_1^\alpha \xi_2^\beta d\Omega = 2A \frac{\alpha! \beta!}{(\alpha + \beta + 2)!}$$

The selection of polynomial terms for
triangles will naturally follow the
Pascal's triangle -

The interpolating triangles are -

3-nodes \rightarrow linear

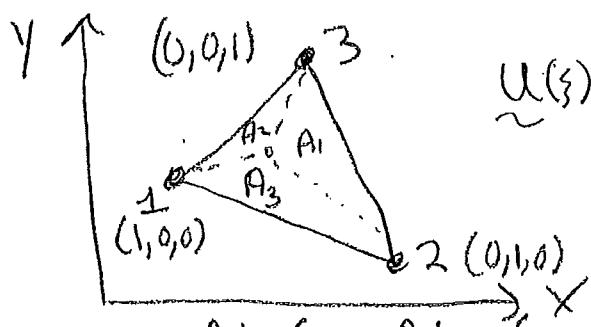
6-nodes \rightarrow quadratic

10-nodes \rightarrow cubic

etc.

3-noded linear, isoparametric triangle

\Rightarrow Directly interpolate using the
area coordinates $N_a = \xi_a = \frac{A_a}{A} \quad a=1(2)3$



$$U(\xi) = \sum_{a=1}^3 N_a(\xi) u_a$$

$$X(\xi) = \sum_{a=1}^3 N_a(\xi) x_a$$

$$N_1 = \xi_1, \quad N_2 = \xi_2, \quad N_3 = \xi_1 + \xi_2 - 1$$

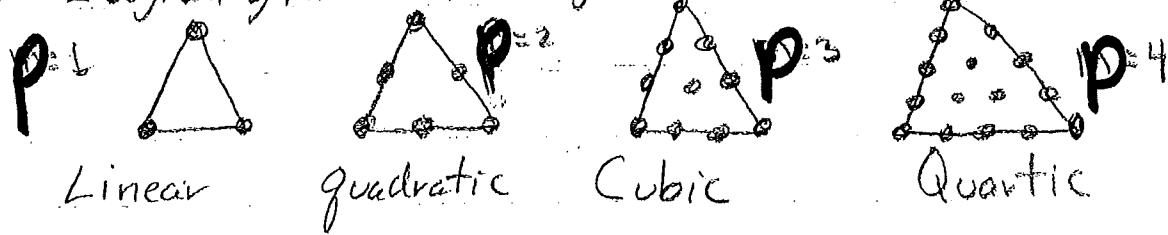
1

 $\xi \eta$ $\xi^2 \xi\eta \eta^2$ $\xi^3 \xi^2\eta \xi\eta^2 \eta^3$ $\xi^4 \xi^3\eta \xi^2\eta^2 \xi\eta^3 \eta^4$ $\xi^5 \xi^4\eta \xi^3\eta^2 \xi^2\eta^3 \xi\eta^4 \eta^5$ $\xi^6 \xi^5\eta \xi^4\eta^2 \xi^3\eta^3 \xi^2\eta^4 \xi\eta^5 \eta^6$ $\xi^7 \xi^6\eta \xi^5\eta^2 \xi^4\eta^3 \xi^3\eta^4 \xi^2\eta^5 \xi\eta^6 \eta^7$

Interpolation for triangles

Note: Expressions differ in notation from that in Appendix 3.I, result the same

For Lagrangian triangles



complete polynomials of order P

have $n = P+1$ nodes on each side

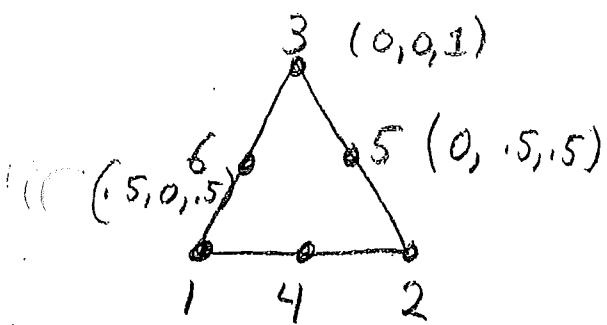
$$U = \sum_{a=1}^{n_{en}} N_a \text{data} \quad n_{en} = \frac{1}{2} (P+1)(P+2)$$

of terms in complete
 P^{th} order polynomial

$$N_a(\xi) = \prod_{i=1}^3 N_i^a(\xi_i) = N_1^a(\xi_1) N_2^a(\xi_2) N_3^a(\xi_3)$$

$$N_i^a(\xi_i) = \begin{cases} \prod_{j=1}^{I_i^a} \left(\frac{\xi_i - J+1}{J} \right) & \text{for } I_i^a \geq 1 \\ 1 & \text{for } I_i^a = 0 \end{cases}$$

$$I_i^a = P \xi_i / \xi_a \quad \leftarrow \text{Second term is the } \xi_i \text{ coordinate of node } a$$



$p=2$

T-11

$$N_1 = N_1'(\xi_1) N_2'(\xi_2) N_3'(\xi_3)$$

$$I_1^1 = 2(1)=2, I_2^1 = 2(0)=0, I_3^1 = 2(0)=0$$

$$(1,0,0) \quad (.5,.5,0) \quad (0,1,0)$$

$$N_2^1 = 1, N_3^1 = 1, N_1^1 = \frac{2}{\pi} \left(\frac{2\xi_1 - T+1}{T} \right) = 2\xi_1 \left(\frac{2\xi_1 - 1}{2} \right) = \xi_1(2\xi_1 - 1)$$

$$N_1 = \xi_1(2\xi_1 - 1)(1)(1) = \xi_1(2\xi_1 - 1)$$

$$N_2 = N_1^2(\xi_1) N_2^2(\xi_2) N_3^2(\xi_3), I_1^2 = 0 = I_3^2, I_2^2 = 2$$

$$N_1^2 = 1, N_3^2 = 1, N_2^2 = \frac{2}{\pi} \left(\frac{2\xi_2 - T+1}{T} \right) = \xi_2(2\xi_2 - 1)$$

$$N_2 = (1) \xi_2(2\xi_2 - 1)(1) = \xi_2(2\xi_2 - 1)$$

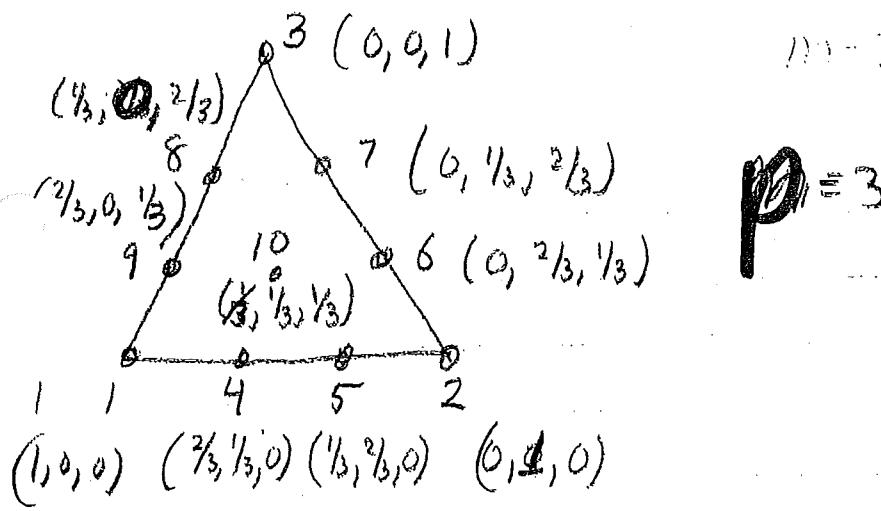
$$N_4 = N_1^4(\xi_1) N_2^4(\xi_2) N_3^4(\xi_3), I_1^4 = 2(.5) = 1, I_2^4 = 2(.5) = 1, I_3^4 = 0$$

$$N_1^4 = \frac{1}{\pi} \left(\frac{2\xi_1 - T+1}{T} \right) = 2\xi_2, N_2^4 = 2\xi_2, N_3^4 = 1$$

$$N_4 = 4\xi_1 \xi_2$$

T=12

M=3



$$N_1 = N_1^1(\xi_1) N_2^1(\xi_2) N_3^1(\xi_3), I_1^{(1)} = 3(1) = 3, I_2^{(1)} = 0, I_3^{(1)} = 0$$

$$N_1^1 = \frac{3}{\pi} \left(\frac{3\xi_1 - J+1}{J} \right) = 3\xi_1 \left(\frac{3\xi_1 - 1}{2} \right) \left(\frac{3\xi_1 - 2}{3} \right) = \frac{\xi_1}{2} (3\xi_1 - 1)(3\xi_1 - 2)$$

$$N_1 = N_1^1 \cancel{N_2^1} \cancel{N_3^1} = \frac{\xi_1}{2} (3\xi_1 - 1)(3\xi_1 - 2)$$

$$N_4 = N_1^4(\xi_1) N_2^4(\xi_2) N_3^4(\xi_3), I_1^{(4)} = 3(2/3) = 2, I_2^{(4)} = 3(1/3) = 1, I_3^{(4)} = 0$$

$$N_1^4 = \frac{2}{\pi} \left(\frac{3\xi_1 - J+1}{J} \right) = 3\xi_1 \left(\frac{3\xi_1 - 1}{2} \right) \cancel{N_3^4 = 1}$$

$$N_2^4 = \frac{1}{\pi} \left(\frac{3\xi_2 - J+1}{J} \right) = 3\xi_2$$

$$N_4 = 3\xi_1 \left(\frac{3\xi_1 - 1}{2} \right) 3\xi_2 = \frac{9}{2} \xi_1 \xi_2 (3\xi_1 - 1)$$

$$N_{10} = N_1^{10}(\xi_1) N_2^{10}(\xi_2) N_3^{10}(\xi_3), I_2^{(10)} = 3(1/3) = 1$$

$$N_1^{10} = \frac{1}{\pi} \left(\frac{3\xi_1 - J+1}{J} \right) = 3\xi_1$$

$$N_{10} = 3\xi_1 3\xi_2 3\xi_3 = 27 \xi_1 \xi_2 \xi_3$$

Numerical Integration

References:

- Numerical Analysis, Burden and Faires, Pws-Kent Publishing Co.
- F.E. Procedures in Engineering Analysis, R.J.Bathe, Prentice Hall

We have seen that in many cases the integrand to our element stiffness integral is a rational polynomial. Thus we are not likely to have exact integration formulae for the integrals. Thus we must integrate numerically in that case. Even if when we can integrate exactly it is typically more convenient and just as efficient, to use numerical integration.

Numerical quadrature - look at 1-D

$$\int_{-1}^1 g(\xi) d\xi = \sum_{l=1}^{n_{\text{int}}} w_l g(\xi_l) + R_{n_{\text{int}}+1}$$

How do we get to such a formula - will use an interpolating polynomial to approximate the integrand and integrate those terms to obtain our weights. Using our Lagrange polynomials

$$g(\xi) = \sum_{l=1}^{n_{\text{int}}} g(\xi_l) L_l(\xi) + R'$$

in which case our integral becomes

$$\int_{-1}^1 g(\xi) d\xi = \int_{-1}^1 \sum_{l=1}^{n_{int}} g(\xi_l) L_e^{n_{int}-1} d\xi + \int_{-1}^1 R d\xi$$

$$= \sum_{l=1}^{n_{int}} \left(g(\xi_l) \int_{-1}^{n_{int}-1} L_e^{n_{int}-1} d\xi + R \right) = \sum_{l=1}^{n_{int}-1} w_l g(\xi_l) + R_{n_{int}-1}$$

$$\Rightarrow \text{note } w_l = \int_{-1}^{n_{int}-1} L_e^{n_{int}-1} d\xi$$

It can be shown that at some location $-1 \leq \xi \leq 1$

$$\hat{R} = \left(\frac{d g(\xi)}{d \xi^{n_{int}}} \Big|_{\xi=\xi_e} \right) \prod_{l=1}^{n_{int}} (\xi - \xi_l)$$

$$R_{n_{int}-1} = \int_{-1}^1 R d\xi$$

This method is referred to as Newton-Cotes

Two common Newton Cotes rules:

Trapezoidal Rule, $n_{int}=2$, points at $\xi=-1, \xi=1$

$$w_1 = \int_{-1}^1 l_1' d\xi = \int_{-1}^1 \frac{1}{2}(1-\xi) d\xi = 1$$

$$w_2 = \int_{-1}^1 l_2' d\xi = \int_{-1}^1 \frac{1}{2}(1+\xi) d\xi = 1$$

$$R = -\frac{2}{3} g_{avg}(\xi) \leftarrow \text{we will exactly integrate a linear}$$

Simpson's Rule, $n_{int}=3$, points at $\xi=1, \xi=0, \xi=-1$, N-2a

$$w_1 = \int_{-1}^1 l_1^2 d\xi = \int_{-1}^1 \frac{1}{2} (\xi^2 - \xi) d\xi = \frac{1}{3}$$

$$w_2 = \int_{-1}^1 l_2^2 d\xi = \frac{4}{3}, \quad w_3 = \int_{-1}^1 l_3^2 d\xi = \frac{1}{3}$$

The error equation we saw before would say the error is $\propto g_{1333}$

However, a more careful analysis shows that the error is:

$$R = \frac{-89333}{90} (\xi) \quad \leftarrow \text{proportional to } 4^{\text{th}} \text{ derivative}$$

This says cubics would be integrated exactly

How can that be - We only used 3 conditions (values at 3 points) - So why do we do better than $3-1=2$ for the order curve done exactly?

⇒ The positions of the evaluation points in the numerical integration can be used as conditions - In this case one point (the center point) is at an "optimal" location

There is a procedure to find these points which leads to the Gauss Quadrature rule where with n_{int} integration points you can exactly integrate an order $2n_{\text{int}}$ order polynomial

⇒ Note - we are not saying we exactly represent it - just exactly integrate it.

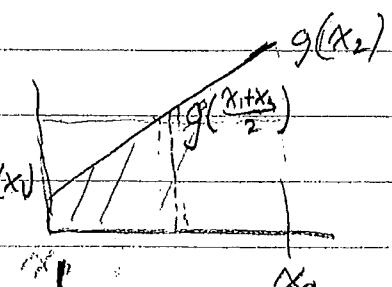
Consider a linear curve -

Can easily exactly integrate using

average value (center value)

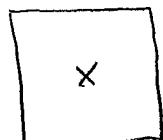
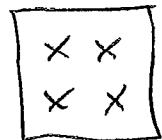
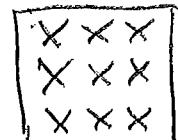
times interval

$$\rightarrow A = g\left(\frac{x_1+x_2}{2}\right) / (x_2 - x_1)$$



Integration for $n_{sd} > 1$

For Quads and Hex elements
Use products of 1-D rules

 1×1  2×2  3×3

$$\int_{-1}^1 \int_{-1}^1 g(\xi, n) d\xi dn \approx \int_{-1}^1 \sum_{i=1}^{n_{int}} g(\xi_i, n) w_i dn$$

$$= \sum_{j=1}^{n_{int,n}} \sum_{j=1}^{n_{int,n}} g(\xi_i, n_j) w_i w_j = \sum_{k=1}^{n_{int}} w_k g(\xi_k, n_k)$$

$n_{int} = n_{int,\xi} n_{int,n}$

For triangles and Tets

A couple of options -

- (A) Degenerate product rule



- (B) Specifically constructed single loop simplex integration rules

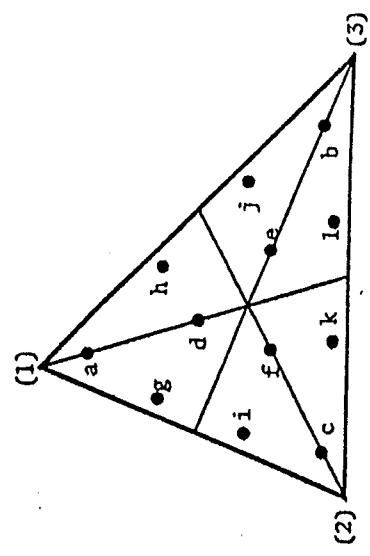
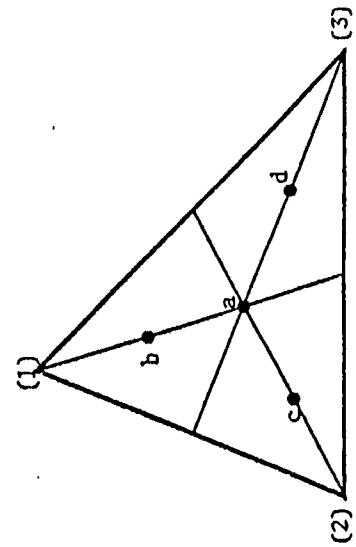
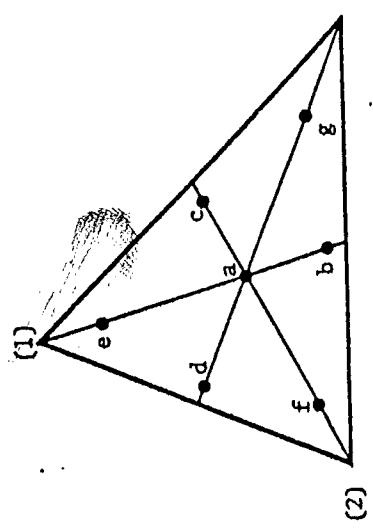
Hammer's Rule Common for triangles

$$\int_{\Delta} g(\xi_1, \xi_2, \xi_3) d\Delta = \sum_{k=1}^{n_{int}} g(\xi_{1k}, \xi_{2k}, \xi_{3k}) w_k$$

SEVEN POINT FIFTH ORDER INTEGRATION

FOUR POINT CUBIC INTEGRATION

TWELVE POINT SIXTH ORDER INTEGRATION



	ξ_1	ξ_2	ξ_3	w
a	1/3	1/3	1/3	9/80
b	α_1	β_1	η_1	
c	β_1	α_1	η_1	
d	β_1	β_1	α_1	
e	α_2	β_2	η_2	
f	β_2	α_2	η_2	
g	β_2	β_2	α_2	

$$\alpha_1 = 0.05971 \quad 58717 \quad 89770$$

$$\beta_1 = 0.47014 \quad 20641 \quad 05115$$

$$\eta_1 = 0.06619 \quad 70763 \quad 94253$$

$$\alpha_2 = 0.79742 \quad 69853 \quad 53087$$

$$\beta_2 = 0.10128 \quad 65073 \quad 23456$$

$$\eta_2 = 0.06296 \quad 95902 \quad 72414$$

	ξ_1	ξ_2	ξ_3	w
a	1/3	1/3	1/3	-27/96
b	3/5	1/5	1/5	25/96
c	1/5	3/5	1/5	25/96
d	1/5	1/5	3/5	25/96

	ξ_1	ξ_2	ξ_3	w
a	α_1	β_1	η_1	
b	β_1	α_1	η_1	
c	β_1	β_1	α_1	
d	α_2	β_2	η_2	
e	β_2	α_2	η_2	
f	β_2	β_2	α_2	
g	α_3	β_3	η_3	
h	β_3	α_3	η_3	
i	β_3	β_3	α_3	
j	α_4	β_4	η_4	
k	β_4	α_4	η_4	
l	β_4	β_4	α_4	

$$\alpha_1 = 0.87382 \quad 19710 \quad 16996$$

$$\beta_1 = 0.06308 \quad 90144 \quad 91502$$

$$\eta_1 = 0.02542 \quad 24531 \quad 85104$$

$$\alpha_2 = 0.50142 \quad 65096 \quad 58179$$

$$\beta_2 = 0.24928 \quad 67451 \quad 70910$$

$$\eta_2 = 0.05839 \quad 31378 \quad 63190$$

$$\alpha_3 = 0.63650 \quad 24991 \quad 21399$$

$$\beta_3 = 0.31035 \quad 24510 \quad 33785$$

$$\eta_3 = 0.05314 \quad 50498 \quad 44816$$

$$\eta_3 = 0.04142 \quad 55378 \quad 09187$$

What order integration should we use?

Remember for generally shaped elements the integrand is a rational polynomial thus our numerical integration can not be exact. Even when the WJ is constant, which means we would have polynomials (with polynomial shape functions) we may not want to integrate exactly.

So if you do not integrate exactly - How well do you want to integrate?

Take this in two levels

First level - Integrate well enough that our F.E. method will execute and, in the limit, converge to the exact solutions

Second level - To integrate well enough that we converge at the maximum possible rate as dictated by the highest order complete polynomial in our shape functions.

First Level - Integrate well enough

Two considerations

- to maintain convergence (minimal rule)
- to avoid a singular stiffness matrix

Maintain Convergence (minimal rule)

(min) Convergence can be maintained so long as we can integrate a constant value of the m^{th} derivative as it appears in the energy inner product correctly

That is we want to correctly integrate

$$\int_{\Omega} c d\Omega = \int_{\square} c |J| d\square = c \int_{\square} |J| d\square$$

Thus we have to integrate the element volume correctly. For polynomial shape functions and isoparametric mappings $|J|$ is a polynomial-

Thus we need to integrate this polynomial exactly to get the correct volume -

This defines a minimum order of integration possible.

To Avoid a Singular Stiffness matrix

we have to solve $\tilde{K} \tilde{d} = \tilde{F}$

if the stiffness matrix is not full rank then \tilde{K}^{-1} does not exist and we can not solve the system

If we use too low an order of numerical integration we will not have sufficient conditions and we would find our elements have too many zero eigenvalues with one or more corresponding to a valid (energy producing if properly integrated eigenmode)

To look at this lets consider 2-D elasticity and individual element matrices

$$\tilde{k}_{\text{ned}}^e$$

Note since the element has no essential boundary conditions an exactly integrated element stiffness matrix will have 3 zero eigenvalues, the eigen vectors of which correspond to the 3 rigid body modes

(C) To Converge at maximum rate possible

For problems with smooth solutions
(no singularities)

$$\|u^h\|_{m+1} \leq \bar{C} h^{k+1-m} \|u\|_{k+1}$$

m - highest order derivative in $a(\omega, u)$

k - highest order complete polynomial in u^h

To maintain the convergence rate
of $k+1-m$ must use a high enough
order integration rule to exactly
integrate a polynomial of order

$$(\bar{k} + k - m)$$

\bar{k} - highest order monomial term
appearing in u^h



$$- \bar{k} = 1$$



$$- \bar{k} = 2 \quad (\text{3 terms})$$



$$- \bar{k} = 2$$

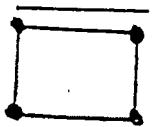
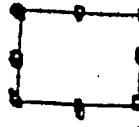


$$- \bar{k} = 3 \quad (3^n \text{ and } 4n^2 \text{ terms})$$



$$- \bar{k} = 4 \quad (3^2 n^2 \text{ terms})$$

ORDERS OF GAUSSIAN INTEGRATION FOR TWO DIMENSIONAL ELEMENTS

ELEMENT	DISPLACEMENT MODEL	GEOMETRY MODEL	USUAL OR	EXACT OR	
			MINIMUM ORDER ^{a,b,c}	RECOMMENDED ORDER ^{a,d}	MAXIMUM ORDER ^d
	LINEAR	RECTANGULAR	1x1	2x2	2x2
	LINEAR	LINEAR	1x1	2x2	3x3
	QUADRATIC	RECTANGULAR	2x2	2x2	3x3
	QUADRATIC	LINEAR	2x2	2x2	3x3
	QUADRATIC	QUADRATIC	3x3	3x3	4x4

- NOTES:
- (A) REFERENCE: ZIENKIEWICZ & HINTON, "REDUCED INTEGRATION, FUNCTION SMOOTHING AND NON-COMFORMITY IN FINITE ELEMENT ANALYSIS", Proc. 1976 INT. CONF. ON FINITE ELEMENT METHODS IN ENGINEERING, UNIV. OF ADELAIDE, AUSTRALIA, AND ALSO TO BE PUBLISHED IN THE JOURNAL OF THE FRANKLIN INSTITUTE,
 - (B) ZERO ENERGY DEFORMATION MODES MAY ARISE. IF THESE ARE NOT INTERELEMENT COMPATIBLE, THEN THE RECOMMENDED ORDER IS HIGHER THAN THE MINIMUM ORDER.
 - (C) BASED ON VOLUME-INTEGRATION CRITERION: COOK, SECTION 5.7
 - (D) REFERENCE: BATHE & WILSON, SECTION 4.7.
 - (E) ALL VALUES GIVEN ARE FOR CONSTANT THICKNESS ELEMENTS.